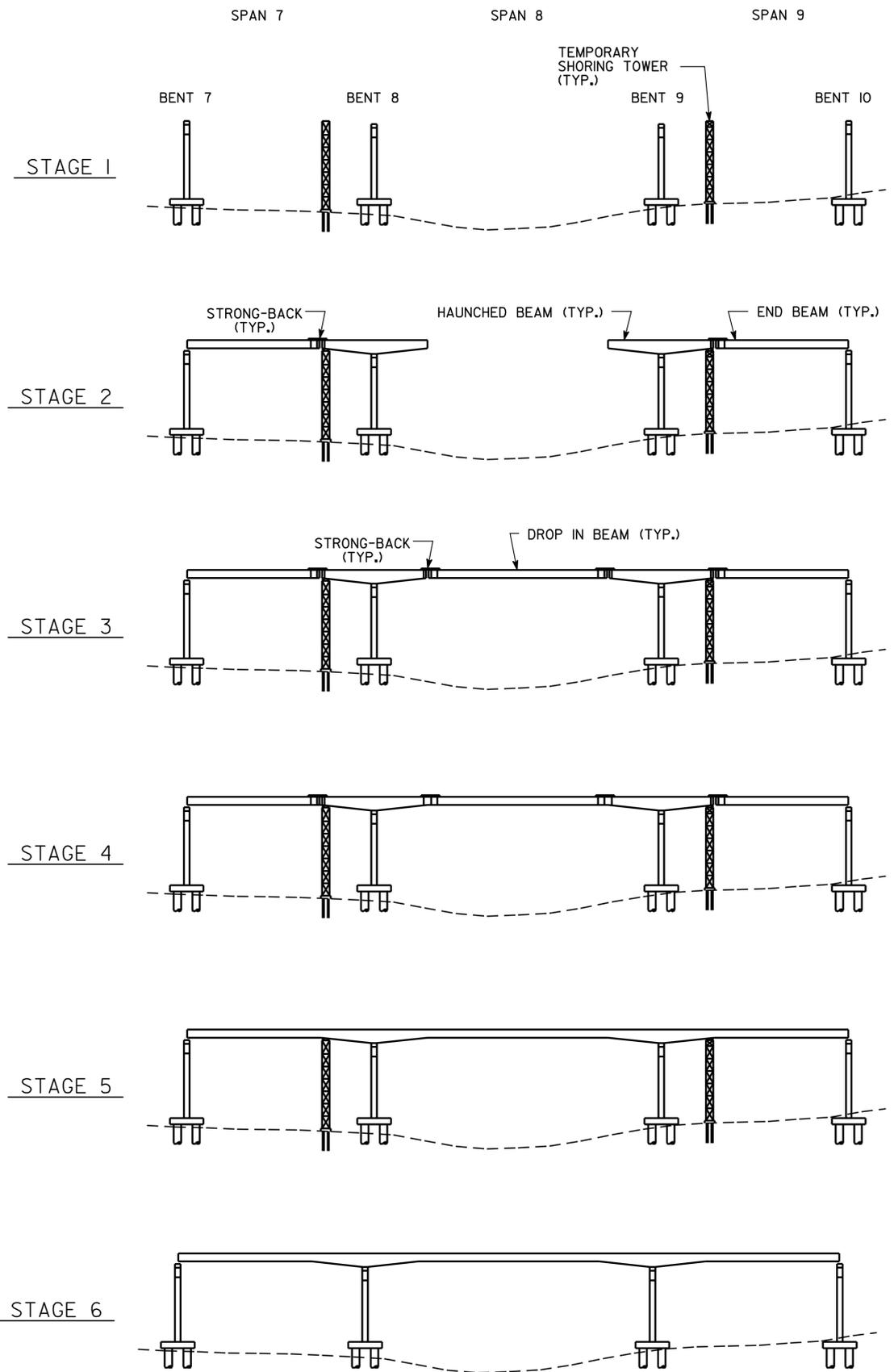


RELEASED FOR CONSTRUCTION 10-05-2010



STAGE 1

1. CONSTRUCT BENT 7 THROUGH 10 AND ERECT TEMPORARY SUPPORTS IN SPANS 7 AND 9.

STAGE 2

1. ERECT HAUNCHED BEAMS AND SECURE TO TEMPORARY SUPPORTS. SECURE HAUNCHED BEAMS TO TEMPORARY SUPPORT AND BENTS 8 AND 9 TO TRANSMIT WIND FORCES ASSUMING THE BEARING PROVIDES NO TRANSVERSE SUPPORT.
2. ERECT END BEAMS AND SECURE TO HAUNCHED BEAMS WITH STEEL STRONG-BACKS. SECURE END BEAMS TO HAUNCHED BEAMS AND BENTS 7 AND 10 TO TRANSMIT WIND FORCES ASSUMING THE BEARING PROVIDES NO TRANSVERSE SUPPORT.
3. THE HAUNCHED BEAMS SHALL BE SECURED TO PREVENT UPLIFT DURING THE PLACEMENT OF THE DROP-IN BEAMS DURING STAGE 3.

AS A MINIMUM, BEAM CONNECTIONS TO TEMPORARY SUPPORT SHALL BE CAPABLE OF TRANSMITTING DEAD LOAD REACTIONS SHOWN IN STAGE 3.

4. STAGE 2 DEAD LOAD REACTIONS ON TEMPORARY SUPPORTS, PER BEAM (EXCLUDING CONSTRUCTION LOADS, WIND LOADS, ERECTION LOADS, IMPACT LOADS AND ANY OTHER MISCELLANEOUS LOADS):

HAUNCHED BEAMS 0 KIPS (UPWARD)

DEAD LOAD REACTIONS ON STRONG-BACKS, PER BEAM (EXCLUDING CONSTRUCTION LOADS, WIND LOADS, ERECTION LOADS, IMPACT LOADS AND ANY OTHER MISCELLANEOUS LOADS):

END BEAMS 105 KIPS (DOWNWARD)

DEAD LOAD REACTIONS SHOWN SHALL BE ADJUSTED FOR ANY CONSTRUCTION LOADS, WIND LOADS, ERECTION LOADS, IMPACT LOADS AND ANY OTHER MISCELLANEOUS LOADS PROPOSED BY THE CONTRACTOR, INCLUDING THE WEIGHT OF THE STRONG-BACKS.

STAGE 3

1. CAST DIAPHRAGMS AT BENTS 8 AND 9 AND PLACE TEMPORARY INTERMEDIATE DIAPHRAGMS.
2. ERECT DROP-IN BEAMS AND SECURE TO HAUNCH BEAMS WITH STEEL STRONG-BACKS. AS A MINIMUM, CONNECTIONS BETWEEN DROP-IN BEAMS AND HAUNCHED BEAMS SHALL BE CAPABLE OF RESISTING RELATIVE TWIST BETWEEN BEAMS.
3. STAGE 3 DEAD LOAD REACTIONS ON TEMPORARY SUPPORTS, PER BEAM (EXCLUDING CONSTRUCTION LOADS, WIND LOADS, ERECTION LOADS, IMPACT LOADS AND ANY OTHER MISCELLANEOUS LOADS):

HAUNCHED BEAMS 123 KIPS (UPWARD)

DEAD LOAD REACTIONS ON STRONG-BACKS, PER BEAM (EXCLUDING CONSTRUCTION LOADS, WIND LOADS, ERECTION LOADS, IMPACT LOADS AND ANY OTHER MISCELLANEOUS LOADS):

END BEAMS 105 KIPS (DOWNWARD)
DROP-IN BEAMS 123 KIPS (DOWNWARD)

DEAD LOAD REACTIONS SHOWN SHALL BE ADJUSTED FOR ANY CONSTRUCTION LOADS, WIND LOADS, ERECTION LOADS, IMPACT LOADS AND ANY OTHER MISCELLANEOUS LOADS PROPOSED BY THE CONTRACTOR, INCLUDING THE WEIGHT OF THE STRONG-BACKS.

4. PLACE REMAINING TEMPORARY INTERMEDIATE DIAPHRAGMS.

STAGE 4

1. STRUCTURAL STEEL TEMPORARY DIAPHRAGM ERECTION SHALL BE COMPLETED BEFORE FALSEWORK OR FORMS ARE PLACED ON THE BEAMS.
2. CAST CLOSURE SPLICE DIAPHRAGMS.

STAGE 5

1. DO NOT BEGIN THIS STAGE UNTIL CONCRETE STRENGTH FOR THE CLOSURE SPLICE REACHES 6500 PSI.
2. LOOSEN BOLTS CONNECTING TEMPORARY INTERMEDIATE DIAPHRAGMS TO BEAMS.
3. FIRST STAGE POST-TENSIONING: AFTER CLOSURE SPLICE DIAPHRAGMS IN SPANS 7 THROUGH 9 HAVE ATTAINED REQUIRED STRENGTH OF 6500 PSI, STRESS FIRST STAGE TENDONS (TENDONS PT-1 AND PT-2).
4. RELEASE BEAMS FROM TEMPORARY SUPPORTS.
5. GROUT FIRST STAGE TENDONS.

ENCASE POST-TENSIONING ANCHORAGES FOR TENDONS PT-1 AND PT-2 WITH EPOXY GROUT POUR-BACK OR APPROVED EQUIVALENT.

6. RETIGHTEN BOLTS CONNECTING TEMPORARY INTERMEDIATE DIAPHRAGMS TO BEAMS.
7. REMOVE STRONG-BACKS.

8. ERECT DECK FORMS IN SPANS 7 THROUGH 9. CAST DECK AND PERMANENT CONCRETE DIAPHRAGMS AS PER APPROVED POURING SEQUENCE.

STAGE 6

1. SECOND STAGE POST-TENSIONING: AFTER THE DECK CONCRETE IN SPANS 7 THROUGH 9 HAS ATTAINED REQUIRED STRENGTH, STRESS SECOND STAGE TENDON (TENDON PT-3).
2. GROUT SECOND STAGE TENDON.

ENCASE POST-TENSIONING ANCHORAGES FOR TENDON PT-3 WITH AN APPROVED TYPE II EPOXY.
3. CAST FINAL PORTION OF DECK AS PER APPROVED POURING SEQUENCE.
4. REMOVE TEMPORARY SUPPORTS IN SPANS 7 AND 9.
5. CAST REMAINING CONCRETE (BARRIERS, ETC.).

NOTES

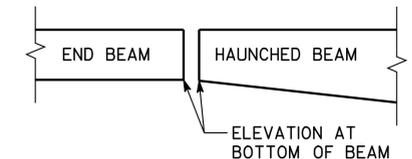
THE CONTRACTOR IS RESPONSIBLE FOR DESIGNING THE TEMPORARY SUPPORTS, STRONG-BACKS AND A CONNECTION THAT WILL PREVENT RELATIVE ROTATION BETWEEN THE DROP-IN BEAMS AND HAUNCH BEAMS. THE DESIGNS SHALL FOLLOW THE AASHTO GUIDE DESIGN SPECIFICATIONS FOR BRIDGE TEMPORARY WORKS, 1995 AND SHALL BE COMPLETED BY A PROFESSIONAL ENGINEER REGISTERED IN GEORGIA. THE CONTRACTOR SHALL SUBMIT SIGNED AND SEALED WORKING DRAWINGS AND CALCULATIONS FOR APPROVAL BY THE ENGINEER.

UPON COMPLETION OF THE CONSTRUCTION OF THE FALSEWORK, INCLUDING STRONGBACKS, SUBMIT THE ENGINEER OF RECORD'S CERTIFICATION THAT THE ACTUAL CONSTRUCTION OF THE FALSEWORK WAS IN COMPLIANCE WITH THE SHOP DRAWINGS THAT WERE STAMPED BY THE DEPARTMENT AND THAT THE MATERIALS AND WORKMANSHIP WAS PROPER AND ADEQUATE FOR THE PURPOSE INTENDED.

THE CONTRACTOR SHALL ENSURE THAT BEARINGS ARE PROPERLY SEATED AND FUNCTIONING AFTER POST-TENSIONING IS COMPLETED.

BEAMS SHALL NOT BE MORE THAN 1/4" OUT OF VERTICAL OR TRANSVERSE ALIGNMENT AT SPLICE LOCATIONS.

FOR DETAILS OF POST-TENSIONING OPERATIONS, SEE POST-TENSIONING NOTES ON DRAWINGS 35-17 AND 35-18.



BOTTOM OF BEAM DIAGRAM
(SHOWING ELEVATION LOCATIONS)

ELEVATIONS OF BOTTOM OF BEAMS AT TEMPORARY SUPPORTS		
BEAM NO.	END BEAM SPAN 7 OR 9	HAUNCHED BEAM SPAN 7 OR 9
1	71.699	71.580
2	71.882	71.764
3	72.066	71.947
4	71.882	71.764
5	71.699	71.580

BRIDGE NO. 1

		THE LPA GROUP INCORPORATED TRANSPORTATION CONSULTANTS 3585 ENGINEERING DRIVE NORCROSS, GEORGIA 30092 (770) 263-9118	
GEORGIA DEPARTMENT OF TRANSPORTATION PRECONSTRUCTION DIVISION-OFFICE OF BRIDGE DESIGN			
CONSTRUCTION SEQUENCE SR 204 SPUR (DIAMOND CAUSEWAY) OVER SKIDAWAY NARROWS CHATHAM COUNTY CSSTP-0008-00(65I)			
SCALE: AS SHOWN		OCTOBER 2010	
DESIGNED: SAD	CHECKED: DGH	REVIEWED: WMD / WEI	
DRAWN: JNA / MDM	DESIGN GROUP: DGH	APPROVED: PVL	

DRAWING NO.	35-19
BRIDGE SHEET	19 OF 48

DATE	
REVISIONS	
BY	